

4) Shear Reinforcement

(P4)

a) For spans AB, BC and CD

Provide minimum shear reinforcement (links) at spans where shear force is minimum or zero. ^{minimum} minimum shear reinforcement here depends on ^{area} of tensile steel.

The minimum amount of tensile steel at any point in the span is 1H10, hence, $A_s = 78 \text{ mm}^2$

$$\frac{100A_s}{bd} = \frac{100 \times 78}{250 \times 456} = 0.068$$

Nominal shear stress, τ_c (Table 3.8, BS 8110) = 0.34 N/mm^2

Since $f_{cu} = 30 \text{ N/mm}^2$ assumed here,

$$\text{Actual } \tau_c = 0.34 * \left[\frac{30}{25} \right]^{1/3} = 0.36 \text{ N/mm}^2$$

Provide minimum links with spacing $S_{lc} \leq 0.75d = 342$

Provide H8 link with spacing = 300 mm c/c

$$\frac{A_{sv}}{S_{lc}} = \frac{0.46}{0.87 f_y}$$

b) Support A

critical section for shear can be taken at a distance 'd' from the face of the support i.e. $0.2 + 0.456 = 0.656 \text{ m}$ from the centre of end support A

$$\text{Design shear force } V = 42 - 0.656w = 42 - 0.656 \times 20 = 29 \text{ kN}$$

$$\text{Design shear stress, } \tau_c = \frac{V}{bd} = \frac{29 \times 10^3}{250 \times 456} = 0.25 \text{ N/mm}^2$$

$$\frac{100A_s}{bd} = \frac{100 \times 226/2}{250 \times 456} = 0.1$$